

CAD/CAM (21-342)

*Advanced Manufacturing Laboratory
Department of Industrial Engineering
Sharif University of Technology*

Session # 9



Course Description

■ *Instructor*

- *Omid Fatahi Valilai, Ph.D. Industrial Engineering Department, Sharif University of Technology*
- *Email: FValilai@sharif.edu, Tel: 6616-5706*
- *Website: Sharif.edu/~fvalilai*

■ *Class time*

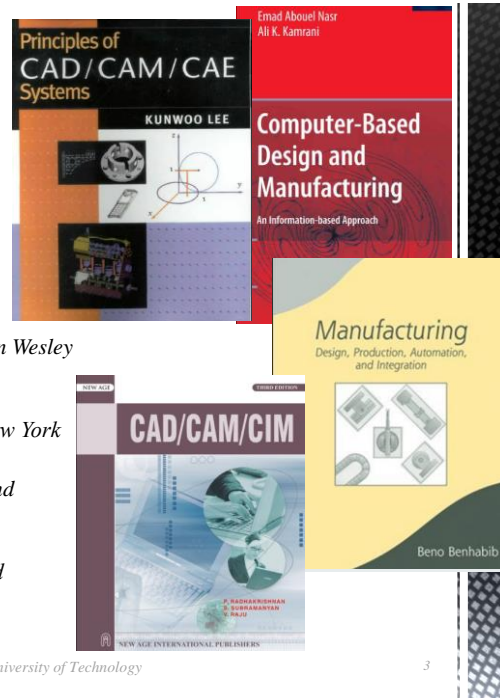
- *Saturday- Monday 10:30-12:00*

■ *Course evaluation*

- *Mid-term (25%)*
- *Final exam (40%)*
- *Quiz (5%)*
- *Exercise (30%)*

Course Description (Continued ...)

- **Mid-term session:**
 - Monday: 8th Ordibehesht 1393, 10:30 ~ 12:30
- **Final Exam:**
 - Saturday: 24th Khordad 1393, 15:00 ~ 17:30
- **Reference:**
 - Lee, Kunwoo; "Principles of CAD/CAM/CAE systems", 1999, Addison Wesley
 - Abouel Nasr, Emad; Kamrani, Ali K.; "Computer-Based Design and Manufacturing: An Information-Based Approach", 2007, Springer, New York
 - Benhabib, Beno; "Manufacturing: Design, Production, CAD/CAM, and Integration", 2003, Marcel Dekker Inc, New York
 - Radhakrishnan, P.; Subramanian, S.; Raju, V.; "CAD/CAM/CIM", 3rd edition, 2005, New age international (P) limited publishers, New York



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Course Description (Continued..)

- **Contents:**
 - Introduction to CAD/CAM/CAE systems (5 sessions)
 - Components of CAD/CAM/CAE systems (2 sessions)
 - Geometric modeling systems (3 sessions)
 - Optimization in CAD (5 sessions)
 - Rapid prototyping and manufacturing (3 sessions)
 - Virtual engineering (2 sessions)
 - Product Life Cycle Cost Model (2 sessions)
 - Computer-Based Design and Features/Methodologies of Feature Representations (5 sessions)
 - Feature-Based Process Planning and Techniques (3 sessions)
 - Collaborative Engineering (2 sessions)

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Course Description (Continued..)

■ Contents:

■ Optimization in CAD

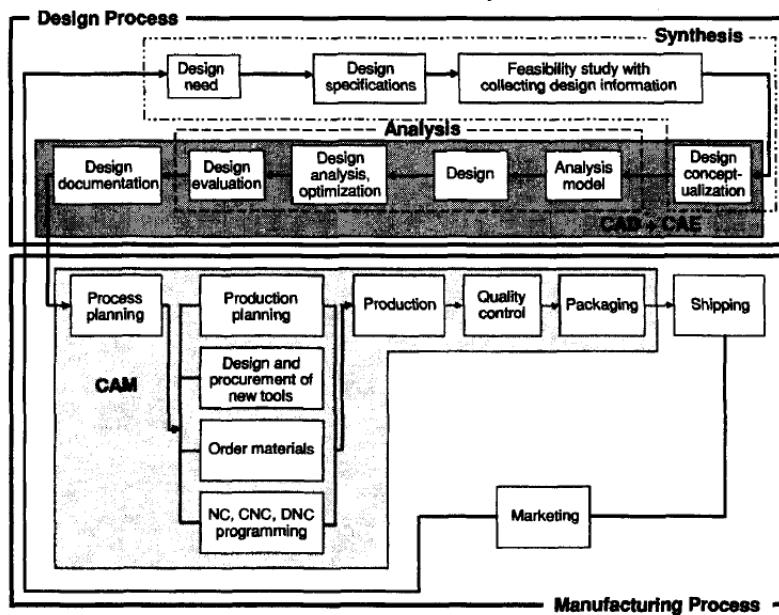
(5 sessions)

- Optimization of optimization problems
- Treatments of constraints
- Search models
- Simulated annealing
- Genetic algorithms
- Structural optimization

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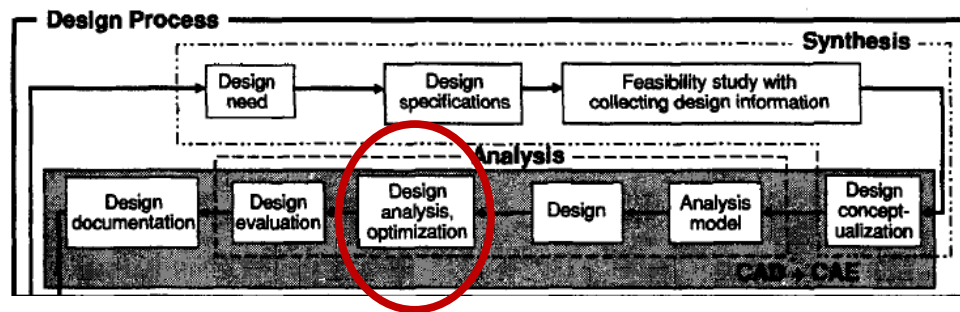
Introduction to CAD/CAM/CAE systems



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Geometric modeling systems

■ Optimization in CAD



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Geometric modeling systems

■ Optimization in CAD

- Design parameterization



Interactive optimization of plate girder bridges subjected to moving loads

H Adeli and K Y Mak

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Program 1
Minimize

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$$y_0(\mathbf{x}) = \sum_{t=1}^{T_0} \sigma_{0t} c_{0t} \prod_{n=1}^N x_n^{a_{0tn}} \quad (5)$$

subject to

$$y_m(\mathbf{x}) = \sum_{t=1}^{T_m} \sigma_{mt} c_{mt} \prod_{n=1}^N x_n^{a_{mnt}} \leq \sigma_m \quad (6)$$

$$x_n > 0 \quad (7)$$

where M is the number of constraints; N is the number of design variables; $m = 1, 2, 3, \dots, M$; $n = 1, 2, 3, \dots, N$; x_n is the n th design variable; T_0 is the number of terms in the objective function; T_m is the number of terms in the m th constraint; $\sigma_{0t} = \pm 1$; $\sigma_{mt} = \pm 1$; and $\sigma_m = \pm 1$. Note that c_{0t} and c_{mt} are the absolute values of coefficients of the t th term in the objective function and the m th constraint respectively. However, a_{0tn} and a_{mnt} are unrestricted in sign.

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For an unstiffened plate girder with a uniform cross section throughout its length, the objective function is written as

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$$W = \rho L [2b_f t_f + h t_w] \quad (11)$$

For a stiffened plate girder, the objective function is

$$W = \rho \left[2b_f t_f L + h t_w L + \alpha h b_s t_s \sum_{i=1}^{NS} N_i \right] \quad (12)$$

where W is the total weight of the plate girder, ρ is the unit weight of steel and L is the total length of the girder. In the case of stiffened plate girders, α distinguishes whether single or double stiffeners are used, and is defined as

$$\alpha = \begin{cases} 1 & \text{for single stiffeners} \\ 2 & \text{for stiffeners used in pairs} \end{cases} \quad (13)$$

The quantity NS is the number of spans, and N_i is the number of intermediate transverse stiffeners in the i th span.

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DESIGN CONSTRAINTS

Design for maximum design loads

For homogeneous plate girders, the requirement for maximum moment strength is written in the following GGP form:

$$\frac{6M_u}{F_{yf}} h^{-2} t_w^{-1} + \frac{12M_u}{F_{yf}} t_f h^{-3} t_w^{-1} - 8b_f t_f^3 h^{-3} t_w^{-1} - 6b_f t_f h^{-1} t_w^{-1} - 12b_f t_f^2 h^{-2} t_w^{-1} \leq 1 \quad (14)$$

where F_{yf} is the flange yield stress. For hybrid plate girders, this requirement is written as:

$$\begin{aligned} & \frac{3M_u h}{4F_{yf} b_f t_f^3} + \frac{M_u h^2 t_w}{8F_{yf} b_f^2 t_f^4} + \frac{3M_u}{2F_{yf} b_f t_f^2} + \frac{M_u h t_w}{4F_{yf} b_f^2 t_f^3} \\ & - \left[\frac{\rho}{32} - \frac{\rho^3}{96} \right] \frac{h^4 t_w^2}{b_f^2 t_f^4} - [3\rho - \rho^3] \frac{h^2 t_w}{8b_f t_f^2} \\ & - [2 + 3\rho - \rho^3] \frac{h^3 t_w}{16b_f t_f^3} - \left[\frac{\rho}{4} - \frac{\rho^3}{12} \right] \frac{h t_w}{b_f t_f} \\ & - \frac{3h}{2t_f} - \frac{3h^2}{4t_f^2} \leq 1 \end{aligned} \quad (15)$$

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DESIGN CONSTRAINTS

Design for service loads

A bridge must be designed to withstand repetitive loading and potential for fatigue. The service load combination in the AASHTO specifications is intended for this purpose.

For unstiffened plate girders, the flexural stress range at the outermost fibre of the I section is checked. This requirement translates into the following

$$\frac{6M_r}{F_{sr} h^2 t_w} + \frac{12M_r t_f}{F_{sr} h^3 t_w} - \frac{8b_f t_f^3}{h^3 t_w} - \frac{6b_f t_f}{h t_w} - \frac{12b_f t_f^2}{h^2 t_w} \leq 1 \quad (32)$$

where M_r is the maximum bending moment range due to truck or lane loading and F_{sr} is the allowable flexural

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A structural optimisation technique

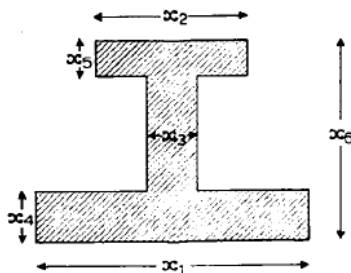
J. A. Ellis
P. E. West

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Optimum Design of a Simply Supported Pre-Tensioned Concrete Roof Purlin

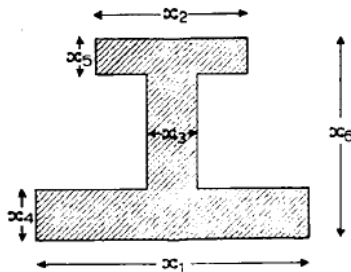
The problem which we wish to solve can be stated in the following form:

A purlin is to be designed as economically as possible for a span of L ft. and a load of w lb./ft. It is to be precast in a factory, and is designed to be handled at any point along its length during transport and erection. The permissible stresses at handling and under working load are given for both tension and compression. Also given are:

- (a) Concrete cover required at both top and bottom flanges.
- (b) The cost of concrete and steel.
- (c) The maximum permitted overall depth of the beam.
- (d) The safety factor for the ultimate moment and ultimate shearing force.
- (e) The U.T.S. and dimensions of the tensioning steel to be used.
- (f) The loss ratio for the prestressing force.
- (g) The horizontal and vertical separation required between the prestressing wires.

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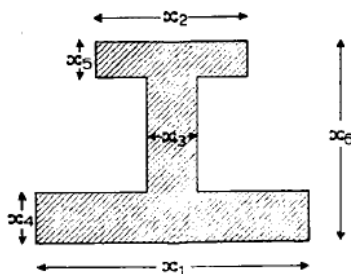
The object of the design procedure is to design that beam for which the total cost of materials is minimised and which satisfies the following constraints:

- (1) The maximum permissible compressive stress in the bottom flange must not be exceeded during handling.
- (2) The maximum permissible tensile stress in the top flange must not be exceeded during handling.
- (3) The maximum permissible tensile stress in the bottom flange must not be exceeded under working load.
- (4) The maximum permissible compressive stress in the top flange must not be exceeded under working load.

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- (5) The ultimate shear stress in the web must not exceed the maximum tensile stress permitted. Symbolically the following five constraints must hold:

$$f_{ct} + \frac{M_{min}}{Z_1} - \frac{P_t}{A} + \frac{P_t e_s}{Z_1} > 0$$

$$\frac{P_t}{A} + \frac{P_t e_s}{Z_2} + \frac{M_{min}}{Z_2} - f_{mint} > 0$$

$$R_0 \left(\frac{P_t}{A} - \frac{P_t e_s}{Z_1} \right) - \frac{M_w}{Z_1} - f_{minw} > 0$$

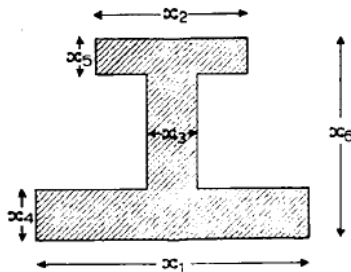
$$f_{cw} - \frac{M_w}{Z_2} - R_0 \left(\frac{P_t}{A} + \frac{P_t e_s}{Z_2} \right) > 0$$

$$\frac{f_h}{2} - \frac{1}{2} \sqrt{(f_h^2 + 4f_{sh}^2)} + C_t > 0$$

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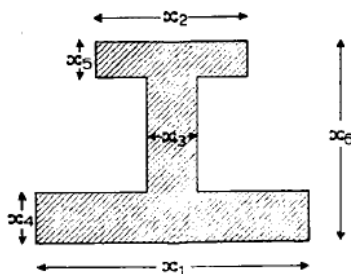
- (6) The overall depth shall not exceed that permitted,

$$x_8 < D_{max}$$

- (7) The geometry of the beam shall be practical, that is the sum of the depth of the flanges shall not exceed the overall depth.
- (8) The depth of the flanges shall be greater than a certain fraction of their width.
- (9) The concrete cover at any wire shall not be less than that specified.
- (10) The horizontal and vertical separation of the wires shall be that specified.
- (11) The numbers of wires in top and bottom flanges shall be integral.
- (12) The dimensions of the section variables will be given to the nearest $\frac{1}{4}$, $\frac{1}{2}$ or 1 in. as required.
- (13) The reinforcement in the bottom flange will be sufficient for ultimate moment considerations.

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